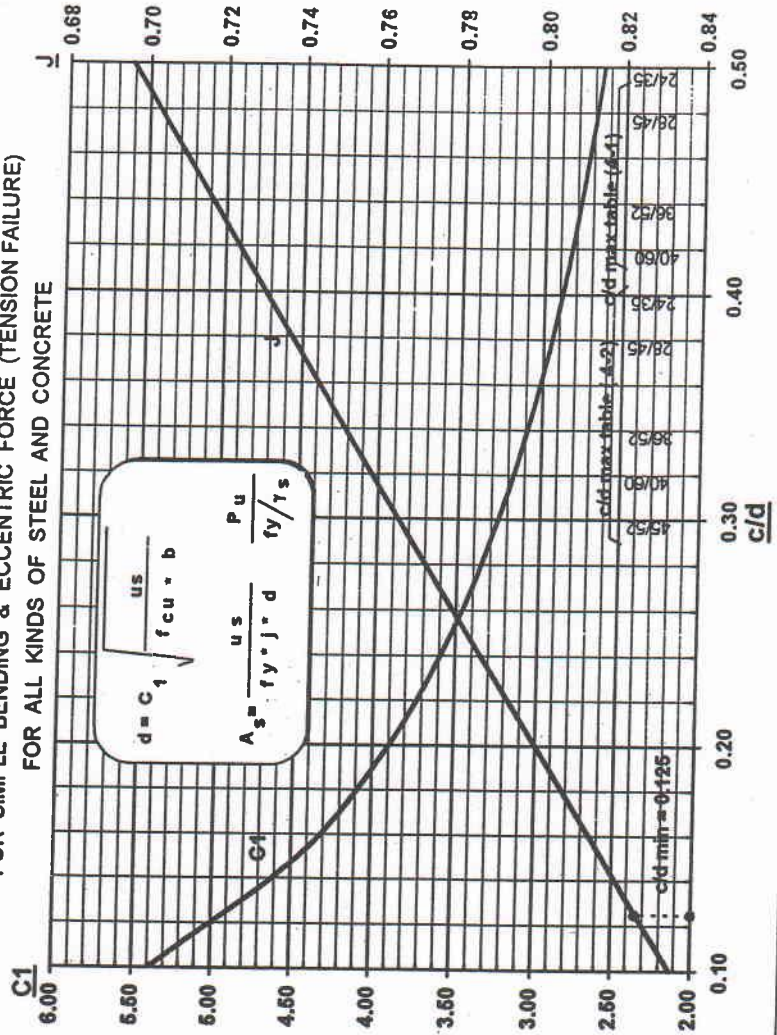


ULTIMATE LIMIT DESIGN CHARTS

FOR SIMPLE BENDING & ECCENTRIC FORCE (TENSION FAILURE)
FOR ALL KINDS OF STEEL AND CONCRETE



c/d	C1	J
0.1250	4.854	0.826
0.1375	4.640	0.821
0.1500	4.455	0.817
0.1625	4.291	0.813
0.1750	4.146	0.808
0.1875	4.016	0.804
0.2000	3.899	0.800
0.2125	3.793	0.796
0.2250	3.697	0.791
0.2375	3.608	0.786
0.2500	3.526	0.782
0.2625	3.451	0.778
0.2750	3.381	0.773
0.2875	3.316	0.769
0.3000	3.255	0.766
0.3125	3.199	0.760
0.3250	3.146	0.756
0.3375	3.096	0.752
0.3500	3.049	0.747
0.3625	3.004	0.743
0.3750	2.963	0.739
0.3875	2.923	0.734
0.4000	2.885	0.730
0.4125	2.850	0.726
0.4250	2.816	0.721
0.4375	2.784	0.717
0.4500	2.753	0.713
0.4625	2.724	0.708
0.4750	2.696	0.704
0.4875	2.670	0.700
0.5000	2.645	0.695

$$C_1 = \frac{1}{\sqrt{0.3573 \cdot c/d (1 - 0.4c/d)}}$$

$$J = \frac{1}{\sqrt{1 - 0.4c/d}}$$

El-Behairy R.C. Design handbook (Design charts)

Chart (C)

VII.1. LOAD DISTRIBUTION ON TWO WAY SLABS

1. Load Distribution According to U.A.R.

r	1.00	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.00
α	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85
β	0.35	0.29	0.25	0.21	0.18	0.16	0.14	0.12	0.11	0.09	0.08

2. Load Distribution According to Marcus :

r	1.00	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.00
α	.396	.473	.543	.606	.660	.706	.746	.778	.806	.830	.849
β	.396	.323	.262	.212	.172	.140	.113	.093	.077	.063	.053

3. Load Distribution According to Grashoff :

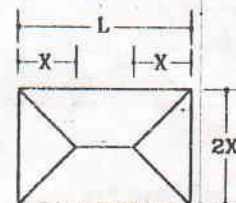
r	1.00	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.00
α	.500	.595	.672	.742	.797	.834	.869	.893	.914	.928	.941
β	.500	.405	.328	.258	.203	.166	.131	.107	.086	.072	.059

$$r = \frac{m.b}{m.a} ; \text{ where } m = 0.87 \text{ for continuity at one end of the slab}$$

$$= 0.76 \text{ for continuity at both ends of the slab}$$

VII.2. EQUIVALENT LOAD FOR DESIGN OF BEAMS :

supporting two way Slabs :



$\frac{L}{2x}$	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0
α	.667	.725	.769	.803	.829	.852	.870	.885	.897	.908	.917
β	.500	.545	.583	.615	.643	.667	.688	.706	.722	.737	.750

El-Beairy R.C. Design Handbook Chapter (7) Load Distribution

Area of Steel Bars in cm² (used in Egypt)

Φ mm	Weight kg/m'	Cross sectional area (cm ²)											
		1	2	3	4	5	6	7	8	9	10	11	12
6	0.222	0.28	0.57	0.85	1.13	1.41	1.70	1.98	2.26	2.54	2.83	3.11	3.39
8	0.395	0.50	1.01	1.51	2.01	2.51	3.02	3.52	4.02	4.52	5.03	5.53	6.03
10	0.617	0.79	1.57	2.36	3.14	3.93	4.71	5.50	6.28	7.07	7.85	8.64	9.42
12	0.888	1.13	2.26	3.39	4.52	5.65	6.79	7.92	9.05	10.18	11.31	12.44	13.57
14	1.208	1.54	3.08	4.62	6.16	7.70	9.24	10.78	12.32	13.85	15.39	16.93	18.47
16	1.578	2.01	4.02	6.03	8.04	10.05	12.06	14.07	16.08	18.10	20.11	22.12	24.13
18	1.998	2.54	5.09	7.63	10.18	12.72	15.27	17.81	20.36	22.90	25.45	27.99	30.54
20	2.466	3.14	6.28	9.42	12.57	15.71	18.85	21.99	25.13	28.27	31.42	34.56	37.70
22	2.984	3.80	7.60	11.40	15.21	19.01	22.81	26.61	30.41	34.21	38.01	41.81	45.62
25	3.853	4.91	9.82	14.73	19.63	24.54	29.45	34.36	39.27	44.18	49.09	54.00	58.90
28	4.834	6.16	12.32	18.47	24.63	30.79	36.95	43.10	49.26	55.42	61.58	67.73	73.89
32	6.313	8.04	16.08	24.13	32.17	40.21	48.25	56.30	64.34	72.38	80.42	88.47	96.51
38	8.903	11.34	22.68	34.02	45.36	56.71	68.05	79.39	90.73	102.1	113.4	124.8	136.1

Area of Other Steel Bars in cm²

Φ mm	Weight kg/m'	Cross sectional area (cm ²)											
		1	2	3	4	5	6	7	8	9	10	11	12
6	0.222	0.28	0.57	0.85	1.13	1.41	1.70	1.98	2.26	2.54	2.83	3.11	3.39
8	0.395	0.50	1.01	1.51	2.01	2.51	3.02	3.52	4.02	4.52	5.03	5.53	6.03
10	0.617	0.79	1.57	2.36	3.14	3.93	4.71	5.50	6.28	7.07	7.85	8.64	9.42
13	1.042	1.33	2.65	3.98	5.31	6.64	7.96	9.29	10.62	11.95	13.27	14.60	15.93
16	1.578	2.01	4.02	6.03	8.04	10.05	12.06	14.07	16.08	18.10	20.11	22.12	24.13
19	2.226	2.84	5.67	8.51	11.34	14.18	17.01	19.85	22.68	25.52	28.35	31.19	34.02
22	2.984	3.80	7.60	11.40	15.21	19.01	22.81	26.61	30.41	34.21	38.01	41.81	45.62
25	3.853	4.91	9.82	14.73	19.63	24.54	29.45	34.36	39.27	44.18	49.09	54.00	58.90
28	4.834	6.16	12.32	18.47	24.63	30.79	36.95	43.10	49.26	55.42	61.58	67.73	73.89
32	6.313	8.04	16.08	24.13	32.17	40.21	48.25	56.30	64.34	72.38	80.42	88.47	96.5
38	8.903	11.34	22.68	34.02	45.36	56.71	68.05	79.39	90.73	102.1	113.4	124.8	136.1