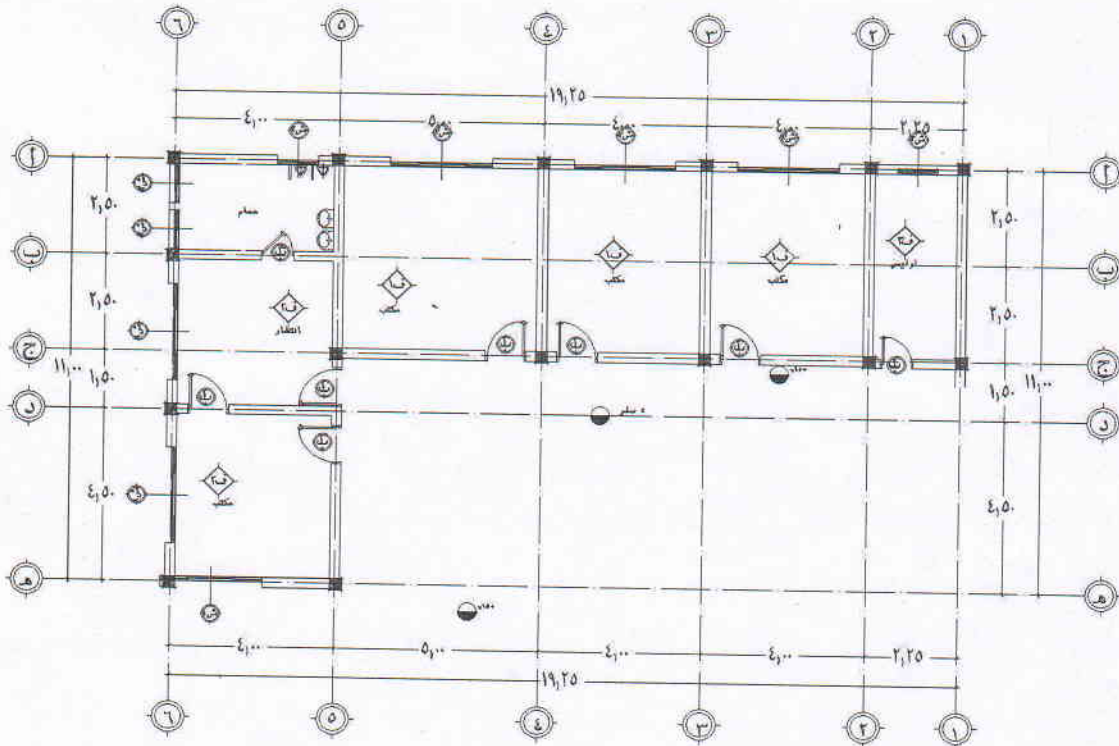


# REINFORCED CONCRETE I

## EXAMMLE 1 :



-Live Load = 200 kg/m<sup>2</sup>

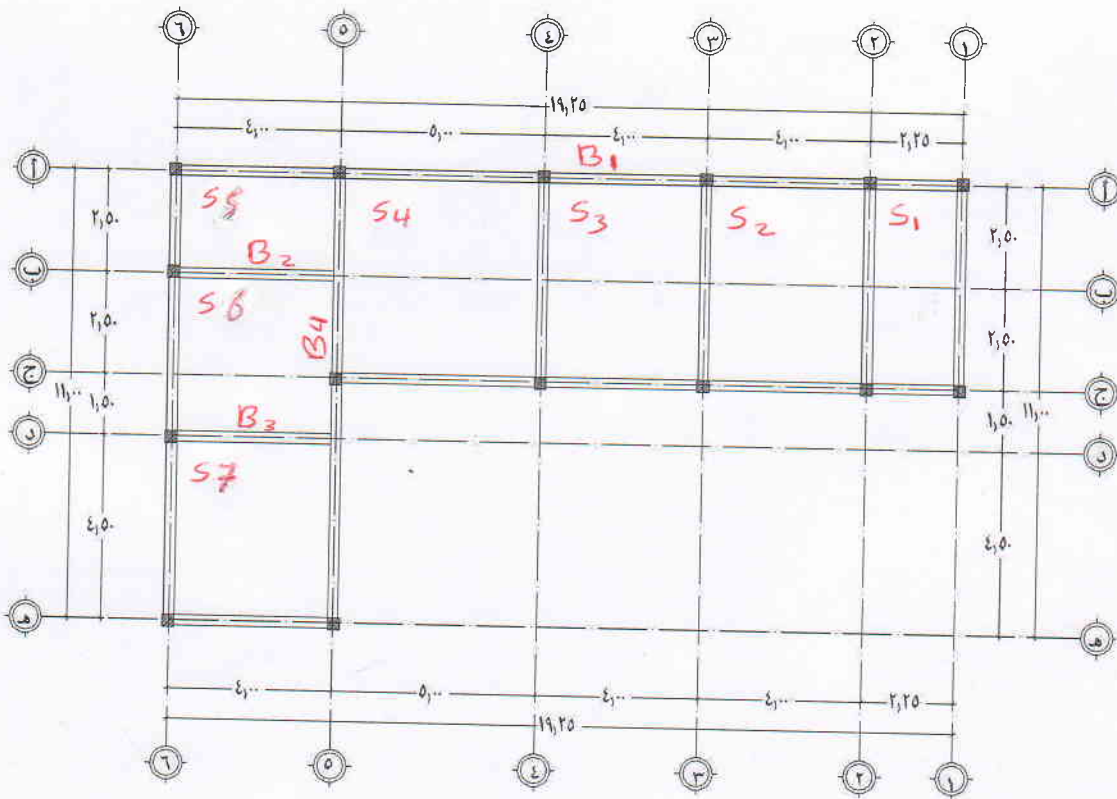
-Flooring = 150 kg/m<sup>2</sup>

-Specific weigh of wall = 1.8 t/m<sup>3</sup>

- Floor Height = 3.50 m

-It is required to find the loads acting on the beam B1,B2,B3,B4.

## - STATICAL SYSTEM:



$$\Rightarrow r_1 = \frac{L_1}{L_2} = \frac{5}{2.25} = 2.22 \Rightarrow \text{oneway} > 2$$

$$\Rightarrow r_2 = \frac{5}{4} = 1.25 < 2 \Rightarrow \text{Two way}$$

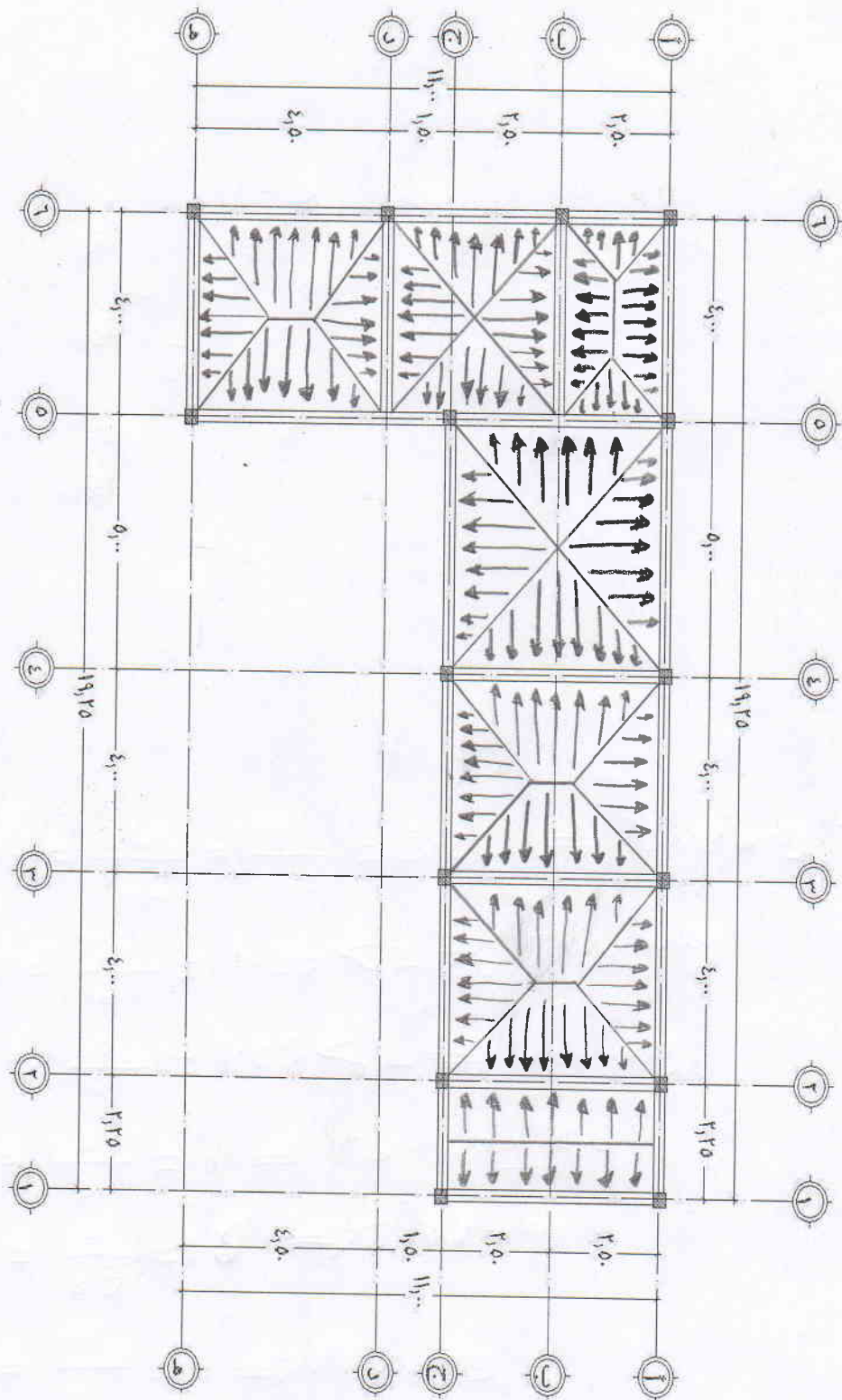
$$\Rightarrow r_3 = \frac{5}{4} = 1.25 < 2 \Rightarrow \text{Two way}$$

$$\Rightarrow r_4 = \frac{5}{5} = 1 \Rightarrow \text{two way} < 2$$

$$\Rightarrow r_5 = \frac{4}{2.5} = 1.6 \Rightarrow \text{two way}$$

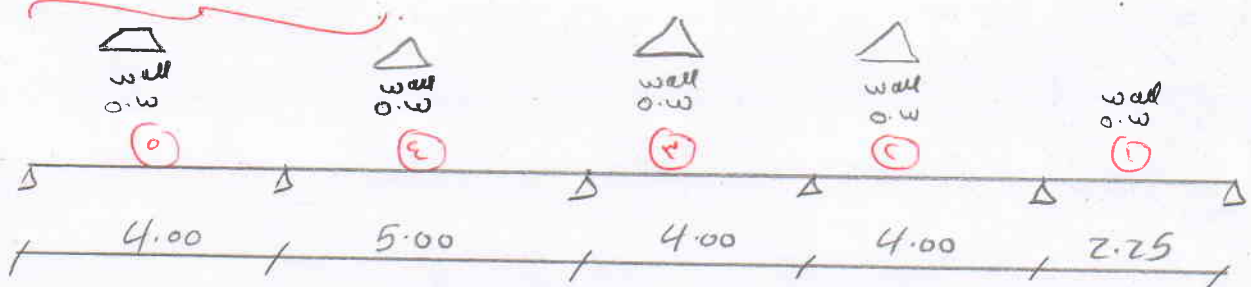
$$\Rightarrow r_6 = \frac{4}{4} = 1 \Rightarrow \text{two way}$$

$$\Rightarrow r_7 = \frac{4.5}{4} = 1.125 < 2 \Rightarrow \text{two way}$$



\* Loads For B<sub>1</sub> :-

\* For moment :-



⇒ Assume that slab thickness is equal = 15 cm

⇒ Assume that the thickness of the beam in the range of (span/10)

Take Beam = (25 X 60) cm

⇒ wall

$$g_{b1} = 0.6 + \text{wall} = b * t * \delta_c + b_w * h_w * \delta_w$$
$$g_{b1} = 0.25 * 0.6 * 2.5 + 0.25 * (3.5 - 0.6) * 1.8$$

$$g_{b1} = 1.68 \text{ t/m}^2$$

(D.L)

$$p_{b1} = \text{zero}$$

(L.L)

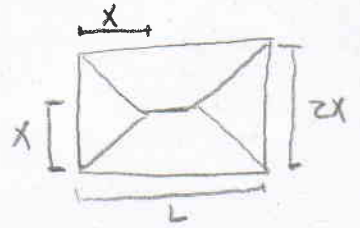
$$w_u = 1.5 (D.L + L.L) = 1.5 (1.68 + 0)$$
$$= 2.52 \text{ t/m}^2$$

$\Rightarrow w_{u2}$

$$g_{b2} = o.w + wall + \triangle = \overbrace{b \times t \times \gamma_c}^{o.w} + \overbrace{b_w \times h_w \times \gamma_w}^{wall} + \triangle \times \gamma_s$$

$$\Rightarrow w_{s1} = 0.15 \times 2.5 + 0.15 = 0.525 \text{ t/m}^2$$

↑  
Flaring



$$g_{b2} = 0.25 \times 0.6 \times 2.5 + 0.25 \times (3.5 - 0.6) \times 1.8 + 0.667 \times 2 \times 0.525$$

$$g_{b2} = 2.38 \text{ t/m}^2$$

$$p_{b2} = \triangle = \alpha \times L \times X = 0.667 \times 0.2 \times 2$$

$$p_{b2} = 0.2668 \text{ t/m}^2$$

$$w_{u2} = 1.5 (g_{b2} + p_{b2}) = 1.5 (2.38 + 0.2668)$$

$$w_{u2} = 3.97 \text{ t/m}^2$$

$\Rightarrow w_{u3}$

$$g_{b3} = o.w + wall + \triangle$$

$$g_{b3} = 0.25 \times 0.6 \times 2.5 + 0.25 \times (3.6 - 0.6) \times 1.8 + 0.667 \times 2 \times 0.525$$

$$g_{b3} = 2.38 \text{ t/m}^2$$

$$p_{b3} = \triangle = \alpha \times L \times X = 0.667 \times 0.2 \times 2$$

$$p_{b3} = 0.2668 \text{ t/m}^2$$

$$w_{u3} = 1.5 (0.2668 + 2.38) = 3.97 \text{ t/m}^2$$

⇒  $w_{u4}$

$$g_{b4} = 0. w + \text{wall} + \triangle$$

$$g_{b4} = 0.25 * 0.6 * 2.5 + 0.25 * (3.5 - 0.6) * 1.8 + 0.667 * 2.5 * 0.525$$

$$g_{b4} = 2.55 \text{ t/m}^2$$

$$p_{b4} = 0.667 * 2.5 * 0.2 = 0.3335 \text{ t/m}^2$$

$$w_{u4} = 1.5 (2.55 + 0.3335) = 4.325 \text{ t/m}$$

⇒  $w_{u5}$

$$g_{b5} = 0. w + \text{wall} + \text{trapezoid}$$

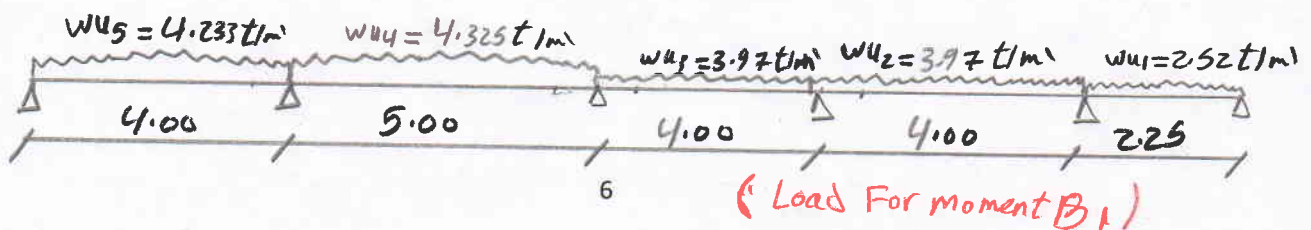
$$g_{b5} = 0.25 * 0.8 * 2.5 + 0.25 * (3.5 - 0.6) * 1.8 + 0.87 * \frac{2.5}{2} * 0.525$$

$$g_{b5} = 2.251 \text{ t/m}^2$$

$$p_{b5} = \text{trapezoid} = \alpha * L.L * \alpha = 0.87 * 0.2 * \frac{2.5}{2}$$

$$p_{b5} = 0.571 \text{ t/m}^2$$

$$w_{u5} = 1.5 (2.251 + 0.571) = 4.233 \text{ t/m}$$



\* Load For shear B<sub>1</sub> !!

⇒  $w_{u1}$

$$g_{s1} = 0. w + wall = 0.25 * 0.6 * 2.5 + 0.25 * (3.5 - 0.6) * 1.8$$

$$g_{s1} = 1.68 \text{ t/m}^1$$

$$P_{s1} = \text{zero}$$

$$w_{u1} = 1.5 (g_{s1} + P_{s1}) = 1.5 (1.68 + 0)$$

$$w_{u1} = 2.52 \text{ t/m}^1$$

⇒  $w_{u2}$

$$g_{s2} = 0. w + wall + \Delta = b * t * d_c + b_w * h_w * d_w + \underbrace{\beta * X * w_s}_{\Delta_{\text{shear}}}$$

$$g_{s2} = \underbrace{0.25 * 0.6 * 2.5}_{0.375} + \underbrace{0.25 * (3.5 - 0.6) * 1.8}_{1.305} + 0.5 * 2 * 0.525$$

$$g_{s2} = 2.205 \text{ t/m}^2$$

$$P_{s2} = \Delta = \beta * X * L.L = 0.5 * 2 * 0.2 = 0.2 \text{ t/m}^2$$

$$w_{u2} = 1.5 (g_{s2} + P_{s2}) = 1.5 (2.205 + 0.2)$$

$$w_{u2} = 3.6075 \text{ t/m}^1$$

$$\Rightarrow \boxed{w_{u3}}$$

$$q_{s3} = 0 \cdot w + w_{\text{wall}} + \triangle$$

$$q_{s3} = 0.25 * 0.6 * 2.5 + 0.25 * (3.6 - 0.6) * 1.8 + 0.5 * 2 * 0.525$$

$$q_{s3} = 2.205 \text{ t/m}^2$$

$$P_{s3} = \triangle = B * X * LL = 0.5 * 2 * 0.2 = 0.2 \text{ t/m}^2$$

$$w_{u3} = 1.5 (2.205 + 0.2) = 3.607 \text{ t/m}$$

$$\Rightarrow \boxed{w_{u4}}$$

$$q_{s4} = 0 \cdot w + w_{\text{wall}} + \triangle$$

$$q_{s4} = 0.25 * 0.6 * 2.5 + 0.25 * (3.5 - 0.6) * 1.8 + 0.5 * 2.5 * 0.525$$

$$q_{s4} = 2.336 \text{ t/m}^2$$

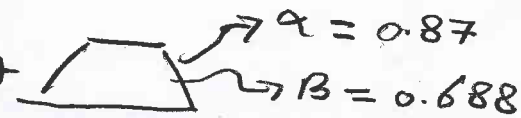
$$P_{s4} = \triangle = B * X * LL = 0.5 * 2.5 * 0.2$$

$$P_{s4} = 0.25 \text{ t/m}^2$$

$$w_{u4} = 1.5 (2.336 + 0.25) = 3.879 \text{ t/m}$$



⇒ WU<sub>5</sub>

$$q_{s5} = 0 \cdot w + \text{wall} + \text{trapezoid}$$


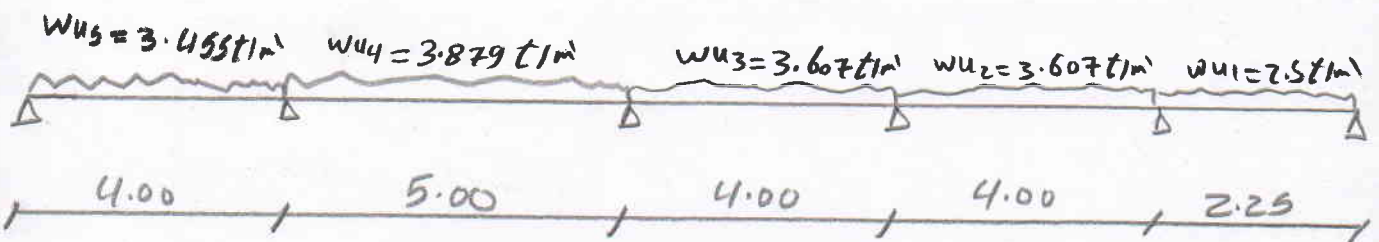
$$q_{s5} = 0.375 + 1.305 + 0.688 * \frac{2.50}{2} * 0.525$$

$$q_{s5} = 2.1315 \text{ t/m}^2$$

$$P_{s5} = \text{trapezoid} = \beta * l * \lambda = 0.688 * 0.2 * 1.25$$

$$P_{s5} = 0.172 \text{ t/m}$$

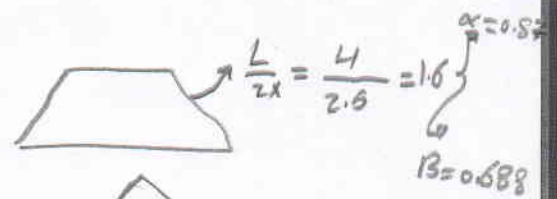
$$WU_5 = 1.5 (0.172 + 2.1315) = 3.455 \text{ t/m}$$



(Load For shear B<sub>1</sub>)

\* Load For  $B_z$  !:

\* For moment !:



$$g_{b1} = 0 \cdot w + \text{wall} + \triangle + \text{trapezoid}$$

$$g_{b1} = b \cdot t \cdot \gamma_c + b_w \cdot h_w \cdot \gamma_w + \alpha \cdot X \cdot w_s + \alpha \cdot X \cdot w_s$$

$$g_{b1} = 0.25 \cdot 0.6 \cdot 2.5 + 0.25 \cdot (3.5 - 0.6) \cdot 1.8 + 0.667 \cdot 2 \cdot 0.525 + 0.87 \cdot 1.25 \cdot 0.525$$

$$g_{b1} = 2.951 \text{ t/m}^2$$

$$p_{b1} = \triangle + \text{trapezoid} = \alpha \cdot X \cdot l \cdot L + \alpha \cdot X \cdot l \cdot L$$

$$p_{b1} = 0.667 \cdot 2 \cdot 0.2 + 0.87 \cdot 1.25 \cdot 0.2$$

$$p_{b1} = 0.4843 \text{ t/m}^2$$

$$w_{u1} = 1.5 (2.951 + 0.4843) = 5.153 \text{ t/m}$$

\* For shear !:

$$g_{s1} = 0 \cdot w + \text{wall} + \triangle + \text{trapezoid}$$

$$g_{s1} = 0.375 + 1.305 + 0.5 \cdot 2 \cdot 0.525 + 0.688 \cdot 1.25 \cdot 0.525$$

$$g_{s1} = 2.6565 \text{ t/m}^2$$

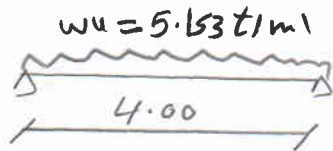
$$p_{s1} = \triangle + \text{trapezoid} = 0.5 + 2 \cdot 0.2 + 0.688 \cdot 1.2 \cdot 0.2$$

$$p_{s1} = 0.365 \text{ t/m}^2$$

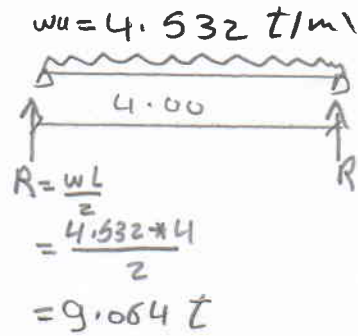
$$w_{u1} = 1.5 (0.365 + 2.6565) = 4.532 \text{ t/m}$$

\* B<sub>2</sub>!!

Load For moment

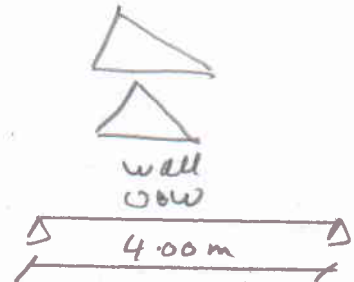


Load For Shear



\* Load For B<sub>3</sub>!!

\* For moment!!



$$q_{b1} = 0.w + wall + \triangle + \triangle$$

$$q_{b1} = 0.375 + 1.305 + \alpha * X * w_s + \alpha * X * w_s$$

$$q_{b1} = 0.375 + 1.305 + 0.667 * 2 * 0.525 + 0.667 * 2 * 0.525$$

$$q_{b1} = 3.08 \text{ t/m}^2$$

$$P_{b1} = \triangle + \triangle = \alpha * X * L + \alpha * X * L = 2 * 0.667 * 2 * 0.2$$

$$P_{b1} = 0.5336 \text{ t/m}^2$$

$$w_{u1} = 1.5 (3.08 + 0.5336) = 5.42 \text{ t/m}$$

\* For shear:

$$q_{s1} = 0 \cdot w + w_{\text{wall}} + \Delta + \Delta$$

$$q_{s1} = 0.375 + 1.305 + B * X * w_s + B * X * w_s$$

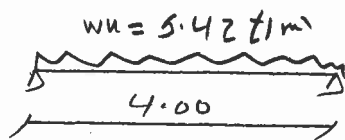
$$q_{s1} = 0.375 + 1.305 + 0.5 * 2 * 0.525 * 2$$

$$q_{s1} = 2.73 \text{ t/m}^2$$

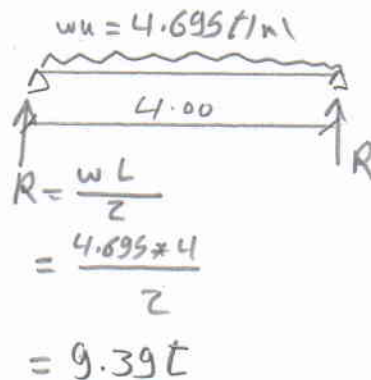
$$P_{s1} = \Delta + \Delta = 2 * 0.5 * 2 * 0.2 = 0.4 \text{ t/m}^2$$

$$w_u = 1.5 (0.4 + 2.73) = 4.695 \text{ t/m}$$

⇒ For moment

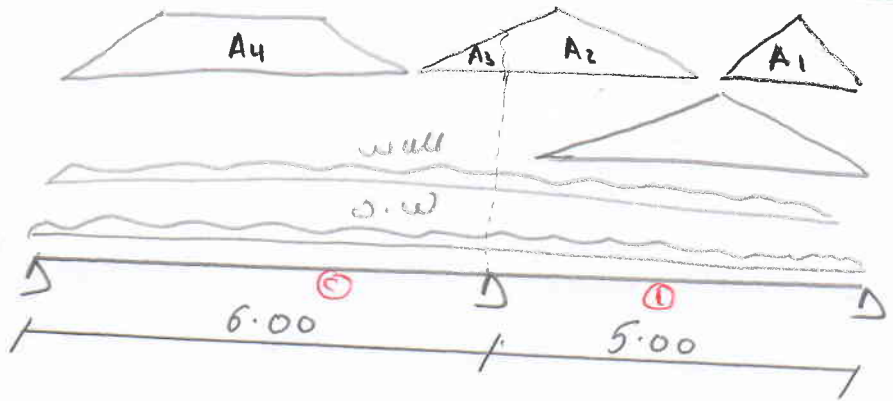


\* For shear



\* For B<sub>y</sub> !:

\* For moment !:



$$A_1 = \frac{2.5 \times 1.25}{2} = 1.5625 \text{ m}^2$$

$$A_2 = \frac{4 \times 2}{2} - \frac{1.5 \times 1.5}{2} = 2.875 \text{ m}^2$$

$$A_3 = \frac{1.5 \times 1.5}{2} = 1.125 \text{ m}^2$$

$$A_4 = \frac{(4.5 + 0.5)}{2} \times 2 = 5 \text{ m}^2$$

$$q_{b1} = \text{o.w} + \text{wall} + \text{triangle}$$

$$q_{b1} = b \times t \times \alpha_c + b_w \times h_w \times \alpha_w + \alpha \times l \times w_s$$

$$q_{b1} = 0.25 \times 0.6 \times 2.5 + 0.25 (35 - 0.8) \times 1.8 + 0.867 \times 2.5 \times 0.525$$

$$q_{b1} = 2.55 \text{ t/m}^2$$

$$p_{b1} = \text{triangle} = \alpha \times l \times \lambda = 0.867 \times 2.5 \times 0.2$$

$$p_{b1} = 0.3335 \text{ t/m}^2$$

$$w_{u1} = 1.5 (0.3335 + 2.55) = 4.32 \text{ t/m}$$

$$q_{bA11} = \frac{w_s \times \text{Area loaded}_1}{\text{loaded length}_1} = \frac{0.525 \times 1.5625}{2.5} = 0.328 \text{ t/m}^2$$

$$P_{bA11} = \frac{L \cdot L \cdot \text{Area loaded}_1}{\text{Loaded Length}_1} = \frac{0.2 \times 1.5625}{2.5} = 0.125 \text{ t/m}^2$$

$$w_{uA11} = 1.5 (0.328 + 0.125) = \boxed{0.6795 \text{ t/m}}$$

$$q_{bA22} = \frac{0.525 \times 2.875}{2.5} = 0.603 \text{ t/m}^2$$

$$P_{bA22} = \frac{0.2 \times 2.875}{2.5} = 0.23 \text{ t/m}^2$$

$$w_{uA22} = 1.5 (0.603 + 0.23) = \boxed{1.2495 \text{ t/m}}$$

$$q_{b2} = 0.0 + w_{\text{wall}} = 0.375 + 1.305 = 1.68 \text{ t/m}^2$$

$$P_{b2} = 0$$

$$w_{u2} = 1.5 (1.68 + 0) = 2.52 \text{ t/m}$$

$$q_{bA33} = \frac{0.525 \times 1.125}{1.5} = 0.393 \text{ t/m}^2$$

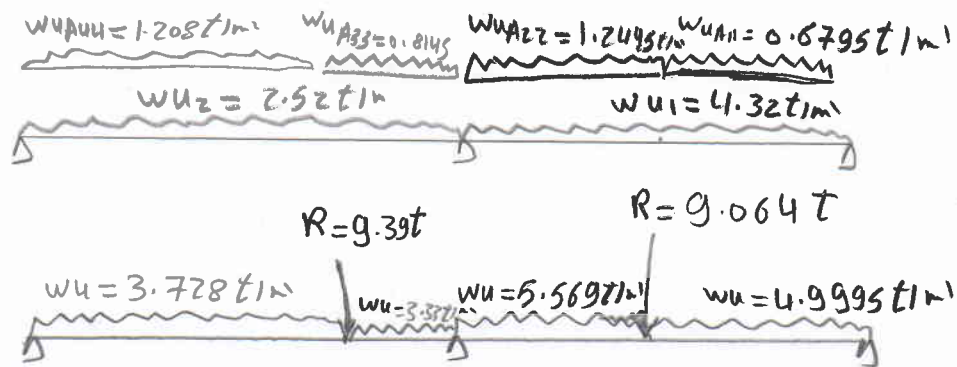
$$P_{bA33} = \frac{0.2 \times 1.125}{1.5} = 0.15 \text{ t/m}^2$$

$$w_{uA33} = 1.5 (0.15 + 0.393) = 0.8145 \text{ t/m}$$

$$q_{bA44} = \frac{0.525 \times 5}{4.5} = 0.583 \text{ t/m}^2$$

$$P_{bA44} = \frac{0.2 \times 5}{4.5} = 0.22 \text{ t/m}^2$$

$$w_{uA44} = 1.5 (0.22 + 0.583) = 1.208 \text{ t/m}$$



For moment

\* For shear :

$$q_{b1} = 0.5w + w_{all} + \triangle = 0.375 + 1.305 + 0.5 \times 2.5 \times 0.925$$

$$q_{b1} = 2.336 \text{ t/m}^2$$

$$p_{b1} = \triangle = 0.5 \times 0.2 \times 2.5 = 0.25 \text{ t/m}^2$$

$$w_{u1} = 1.5 (0.25 + 2.336) = 3.879 \text{ t/m}$$

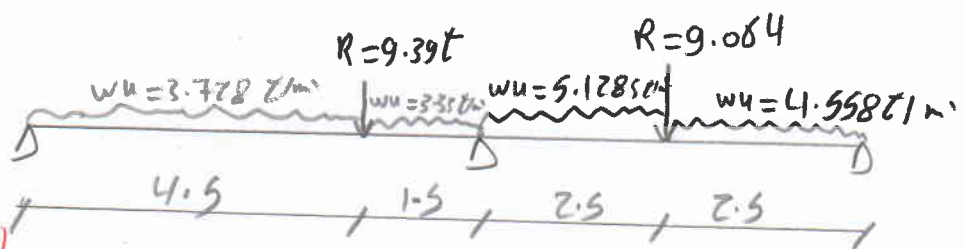
$$w_{uA11} = 0.6795 \text{ t/m}$$

$$w_{uA22} = 1.2495 \text{ t/m}$$

$$w_{uA33} = 0.8145 \text{ t/m}$$

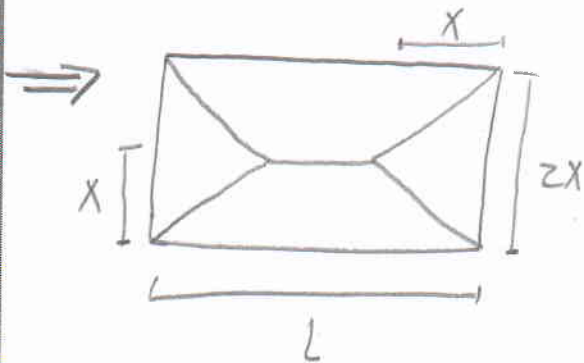
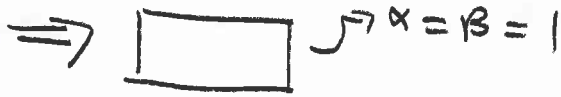
$$w_{uA44} = 1.208 \text{ t/m}$$

$$w_{u2} = 2.52 \text{ t/m}$$

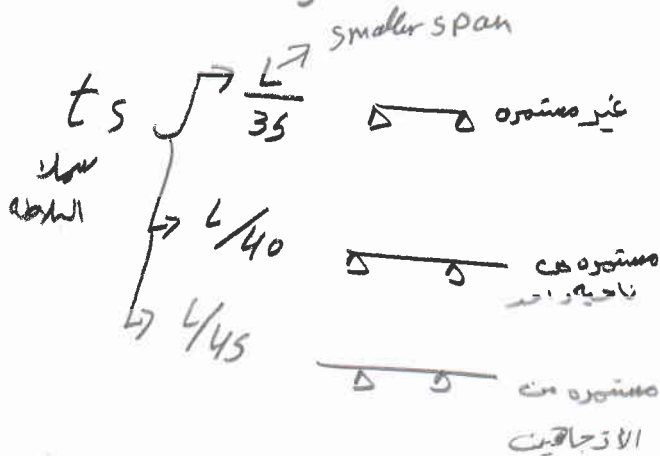


For shear

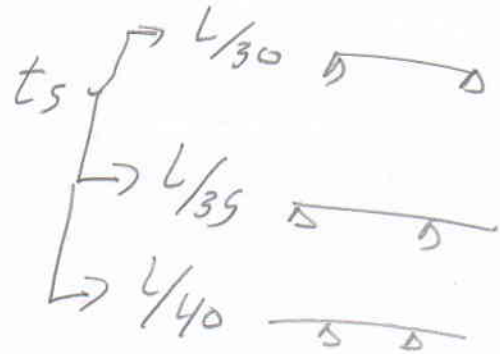
\* لا -> فلا -> ∴



⇒ two way



⇒ one way



⇒ في الحالة الكابولية

$$t_s = \frac{L \rightarrow \text{smaller}}{10}$$

⇒  $w_u = 1.5 (L \cdot L + D \cdot L)$

$L \cdot L < 0.75 D \cdot L$

⇒  $w_u = 1.4 D \cdot L + 1.5 L \cdot L$

$L \cdot L > 0.75 D \cdot L$

$w_s = t_s * \gamma_c + \text{Flooring}$

1 ton = 10 KN